REPORT OF GEOTECHNICAL EXPLORATION FOR BRANDON HYATT HOTEL BRANDON, FLORIDA

Prepared for:

Love Investment Company Tampa, Florida

Prepared by:

MACTEC ENGINEERING AND CONSULTING, INC. Tampa, Florida

December 22, 2008

MACTEC Project 6513-08-0588





engineering and constructing a better tomorrow

December 22, 2008

Love Investment Company

c/o Mr. Don Farris Batson-Cook Company 101 East Kennedy Blvd., Suite 1750 Tampa, FL 33602

Phone: 813-221-7575 Cell: 813-221-7590

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Subject:

Report of Geotechnical Exploration

Brandon Hyatt Hotel Brandon, Florida

MACTEC Project 6513-08-0588

Dear Mr. Farris:

MACTEC Engineering and Consulting, Inc. (formerly known as Law Engineering and Environmental Services), is pleased to submit this report of our geotechnical exploration for the proposed project. Our services were conducted in general accordance with our Proposal TG-08-084 dated December 5, 2008.

The results of our Geotechnical subsurface exploration, including our evaluation of the site and recommendations for site preparation and applicable foundation support systems are presented in this report.

We appreciate the opportunity to be of service to you on this project. Should you have any questions with regard to this report, or if we can be of any further assistance, please contact this office.

Sincerely,

MACTEC ENGINEERING AND CONSULTING, INC.

Bradley M. Johnson, E.I.

Geotechnical Professional

Distribution: 2 - Addressee (mail)

1 – Addressee (e-mail)

1 - File

Curtis J. Roos, P.E. Chief Engineer

Florida Registration 27570

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1.0 EXECUTIVE SUMMARY

The project is located in Brandon, Florida in Hillsborough County. Specifically, the project is located on the southeast corner of the intersection of I-75 and State Road 60. The proposed project consists of the construction of a hotel and restaurant. The anticipated construction includes a six story hotel structure, a single story restaurant and associated parking and driveways. No structural loading information was supplied at this time, however, based on our experience, the maximum expected loads for the hotel structure are 250 kips for a single column and 6 kips per linear foot (klf) for an exterior wall.

The ground surface has veneer of grass. Below the grass, a 4 to 8 foot layer of very loose to dense layers of sand (SP-SM, SM, and SP-SC) were encountered. Below the sand layers in all the borings except A-5 a layer of highly compressible, highly organic peat was encountered. The deepest this material was encountered was at a depth of 15 feet in boring A-2.

In our previous explorations, most of our borings and test pits also encountered a highly compressible soil. This material consisted of black and dark brown highly organic material with root fibers. This material was typically 3.5 to 6 feet thick and was encountered at varying depths in the range of 2 to 8.5 feet.

In order to utilize a shallow foundation system for this site, the organic, highly compressible material will need to be excavated and removed. In its place fill will have to be placed and compacted. Dewatering using wellpoints is recommended to assure complete removal and to facilitate compaction. When properly implemented a shallow foundation system with a bearing capacity of 2.5 ksf will be appropriate.

2.0 INTRODUCTION

2.1 REPORT FORMAT

This report begins with a discussion of the field program, followed by a description of the general subsurface conditions. The site location map is presented on Figure 1, the approximate soil boring locations are presented on Figure 2A, the USDA site vicinity sketch is presented on Figure 3 and the USGS site vicinity sketch is presented on Figure 4. A profile of the borings is shown in Figures 5A and the individual soil boring logs are in the Appendix of this report.

In 2001, MACTEC performed a geotechnical exploration of the northern portion of the site, including five soil test borings, five auger borings and fifteen test pits. Another exploration consisting of six soil test borings was performed on the southern portion of the property in 2004. A third exploration consisting of 2 SPT borings in the northern portion was performed at this site in 2006. The current exploration was conducted in the area of the proposed structures on the southern and eastern side of the property. Included in the Appendix, are the location boring plans (Figures 2B-2E), boring profiles (Figure 5B-5D), boring logs, and test pit records from our previous explorations at the site.

2.2 PROJECT CHARACTERISTICS

The proposed project consists of the construction of a hotel and restaurant. The anticipated construction includes a six story hotel structure with associated parking and driveways. No structural loading information was supplied at this time, however, based on our experience, the maximum expected loads for the hotel are 250 kips for a single column and 6 kips per linear foot (klf) for an exterior wall.

2.3 SITE DESCRIPTION

The project is located in Brandon, Florida in Hillsborough County. Specifically, the project is located on the southeast corner of the intersection of I-75 and State Road 60. The property is a grass covered pasture with several structures including a frame house, and two pole barns, fences and farming equipment. The pasture is currently being used for cattle grazing.

3.0 PURPOSE AND SCOPE

The purpose of this study was to obtain preliminary information on the general subsurface conditions at the proposed project site. The subsurface materials encountered were then evaluated with respect to the available project characteristics. In this regard, engineering assessments for the following items were formulated:

- Identification of the existing ground water levels and estimated normal seasonal high ground water fluctuations.
- General location and description of potentially deleterious materials encountered in the borings, which may result in excessive settlement of the structure.
- Allowable capacities and foundation settlement for foundations supporting the structure.
- Recommendations for foundation design, foundation installation and testing recommendations.
- General site preparation recommendations including structural fill gradation and compaction requirements.
- Suitability of excavated soils for use as fill.

The following services were provided in order to achieve the preceding objectives:

- Performed five soil test borings to a depth of 25 feet in the hotel footprint and one soil test boring to a depth of 20 feet in the restaurant footprint. Samples were collected and Standard Penetration Test resistances were measured at approximate intervals of two feet for the top ten feet and at approximate intervals of five feet thereafter.
- Visually classified and stratified representative soil samples in the laboratory using the Unified Soil Classification System and performed a limited laboratory testing program which includes organic content and grain size analysis from the soil samples obtained in the SPT borings.
- The results of the field exploration were used in our engineering analysis and in the formulation of the recommendations. The results of the subsurface exploration, including recommendations for site preparation and foundation design, are included in this report.

4.0 FIELD EXPLORATION

4.1 GENERAL

Our current field exploration included 5 soil test borings to a depth of 25 feet in the footprint of the proposed hotel footprint and one boring to a depth of 20 feet in the footprint of the proposed restaurant. The boring locations were determined in the field by measuring from existing ground surface features. If more precise locations are desired, we suggest that you contact a Registered Surveyor. The ground surface elevations at the boring locations were neither furnished nor determined. The approximate locations of the borings are illustrated on Figure 2A, which has been included in the Appendix of this report.

Previous explorations include a geotechnical exploration of the northern portion of the site from 2001, which included five soil test borings, five auger borings and fifteen test pits. Another exploration consisting of six soil test borings was performed on the southern portion of the property in 2004. A third exploration consisting of 2 SPT borings in the northern portion was performed at this site in 2006.

The soil test borings were performed with the use of a B-57 Power Drill Rig using a 4 ½ inch diameter hollow stem auger and cathead hammer. The soil sampling was performed in general accordance with ASTM Test Designation D-1686, entitled "Penetration Test and Split-Barrel Sampling of Soils." Samples were obtained at approximate intervals of two feet to a depth of ten feet, and at intervals of five feet thereafter. Representative portions of these soil samples were sealed in glass jars, labeled and transferred to our laboratory for classification by an engineer.

5.0 LABORATORY TESTING

5.1 GENERAL

The soil samples were transported to our laboratory and were classified by the Geotechnical Engineer using the USCS in general accordance with the ASTM Test Designation D-2488. It should be noted that all soil samples will be properly disposed of thirty days following the submittal of the MACTEC subsurface exploration report.

Table 1 summarizes the laboratory test results performed on samples from the current exploration. Table 2 summarizes the laboratory test results from the previous explorations at this site.

Table 1: Laboratory Results from Current Investigation

Boring No.	DEPTH (FT)	MOISTURE CONTENT (%)	PERCENT FINER THAN NO. 200 SIEVE (BY WEIGHT)	ORGANIC CONTENT (%)
A-I	6.5	136.9	-	28.8
A-1	9	14.3	6.0	
A-2	2	24.6	24.9	_
A-3	9	28.9	-	4.7
A-6	4	94.4	~	20.1
A-6	19	58.2	79.6	=

Table 2: Laboratory Results from Previous Explorations

Boring No.	Дертн (FT)	Moisture Content (%)	PLASTIC LIMIT	Liquid Limit	PLASTIC INDEX	PERCENT FINER THAN NO. 200 SIEVE (BY WEIGHT)	ORGANIC CONTENT (%)
B-03	6.5 - 8	17.7					0.9
B-04	1.5 - 3	12.1				6.3	****
B-04	6.5 - 8	40.4	****				6.2
B-05	4 - 5.5	21.3			***	5.2	
B-05	9 - 10.5	25.9		*****			4.3
AB-04	4 - 10	212					37.6
B-1	14.0 – 15.0	59.6	27	182	155	M2 40. A0	va man
B-2	4.0 – 5.5	50.1			****	8.1	7.0
B-4	6.5 - 8.0	205.3					59.8
B-5	6.5 - 8.0	87.0				20-34-54	15.2
SPT-2	24.0 – 25.5	51.2		****		32.6	1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-

6.0 GENERALIZED SUBSURFACE CONDITIONS

6.1 COUNTY SOIL SURVEY

The "Soil Survey of Hillsborough County, Florida," published by the USDA SCS, was reviewed for general near-surface soil information within the general project vicinity (see Figure 3 in the Appendix of this report). This information indicates that there are four primary mapping units, within the proposed project area. The soil map unit characteristics are presented below:

Table 3: Soil Map Unit Characteristics

SOIL SERIES (MAP NUMBER)	SEASONAL HIGH GROUND WATER TABLE DEPTH BELOW NATURAL GRADE (FEET)
Basinger, Holopaw, Samsula (5)	+2.0 - 1.0
St. Johns fine sand (46)	0-1.0
Myakka fine sand (29)	0-1.0
Zolfo (61)	2.0 - 3.6

6.2 USGS TOPOGRAPHY SURVEY

The topographic survey map published by the United States Geological Survey: "Brandon, Florida" dated 1974 (photorevised 1981) was reviewed for ground surface features at the proposed project location (see Figure 4 in the Appendix of this report.) Based on this review, the general ground surface elevation is approximately +30 feet National Geodetic Vertical Datum of 1929 (NGVD).

6.3 GENERAL SUBSURFACE CONDITIONS

The subsurface conditions encountered at the soil test boring locations are described on the Boring Records in the Appendix. These records represent our interpretation of the subsurface conditions based on the field logs, and visual examination of field samples by an engineer. The lines designating the interfaces between various strata on the Boring Records represent the approximate interface locations. In addition, the transitions between strata may be gradual. Water levels shown on the Boring Records represent the conditions only at the time of our exploration. It should be understood that soil and rock conditions may vary between boring locations.

Brandon Hyatt Hotel Brandon, Florida MACTEC Project 6513-08-0588

The ground surface has a veneer of grass. Below the grass, a 4 to 8 foot layer of very loose to dense layer of sand (SP-SM, SM, and SP-SC) was encountered. Below the sand layers in all the borings except A-5 a layer of highly compressible, highly organic peat was encountered. The deepest this material was encountered was at a depth of 15 feet in boring A-2. Below the highly organic material the borings typically encountered layers of sands (SP-SM, SP-SC, and SM) until they reached their termination depths. Boring A-6 encountered green clay (CH and CL) at a depth of 15 feet until the termination depth. It should be noted that in boring A-5 clay similar to that found in A-6 was recovered on the auger from a depth from 16 to 19 feet however was not recovered in the split spoon samples.

In our previous explorations, borings SPT-1, SPT-2, B-2, B-3, B-4 B-5, B-03, B-04, B-05, AB-02, AB-03 and AB-04 encountered a highly compressible soil peat. Test pits TP-4, TP TP-5, TP-6, TP-7, TP-8, TP-9, TP-11, TP-12, TP-13, TP-14, and TP-15 also encountered this material. This material consisted of black and dark brown highly organic material with root fibers. This material was typically 3.5 to 6 feet thick and was encountered at varying depths in the range of 2 to 8.5 feet. Test pit TP-13 encountered a layer 13 feet thick of this material. The N-value was weight of hammer in this stratum. Test pits TP-05 encountered limerock base material 4 inches thick and TP-04 encountered concrete blocks at a depth of two feet.

No "raveled" conditions indicative of "active" sinkhole type activity were encountered during this subsurface exploration program. The profiles of all borings have been included in the Appendix of this report. These profiles illustrate the visual characteristics of all soil strata encountered using the Unified Soil Classification System. Groundwater observations, sampling information and other pertinent field data and observations are also included. A sheet defining the terms and symbols used on the profiles is included in the Appendix of this report.

6.4 GROUND WATER CONDITIONS

Ground water was observed at a depth of approximately 4 to 8.5 feet below the existing grade. It should be noted that ground water levels tend to fluctuate during periods of prolonged drought and extended rainfall and may be affected by man-made influences. In addition, a seasonal effect may also occur during which higher ground water levels are normally recorded during rainy seasons. We anticipate that the seasonal high ground water level will be approximately one foot below the average existing grade.

If the ground water level is critical to design or construction, ground water observation wells should be installed on site to monitor ground water fluctuations over a period of time and to permit more accurate determinations of wet season and dry season levels.

7.0 DESIGN RECOMMENDATIONS

7.1 GENERAL

Our explorations at the site encountered highly compressible soils that will have to be removed prior to construction of the proposed development. Additional fill will also be needed to raise existing grades above the seasonal high ground water level.

Based upon the results of our subsurface explorations, the highly compressible soils were encountered from 4 to 15 feet across the central and southern portion of the site. The highly compressible soils encountered underlying the project site are considered to be unsuitable foundation bearing material.

7.2 FOUNDATION ALTERNATIVES

Our explorations revealed the presence of highly compressible soils located at the project site. These soils are considered unsuitable bearing material for shallow foundations due to excessive settlement potential. In order to reduce the settlement to tolerable levels we recommend one of the following alternatives:

- Excavation and removal of the highly compressible soils, followed by replacement with compacted backfill. Surcharging of the site with temporary fill does not appear to be feasible due to the large variation in peat thickness.
- The use of piles to support the structure and floor slab. Additional deep borings would need to be performed to encounter a pile-bearing layer.

The remainder of this report addresses shallow foundation support after excavation.

7.3 SHALLOW FOUNDATIONS

The highly compressible soils encountered within the proposed project site will need to be excavated, removed and backfilled with approved fill prior to the construction of shallow foundations. As a minimum, the bottom of the excavation should extend a distance 10 feet beyond the building exterior perimeter. Due to the variable depth and content of the deleterious materials, it is considered essential that a representative of the Geotechnical Engineer observe the excavation

process. The excavation should initially extend to a depth of 10 feet. If highly compressible soils are found at the excavation bottom, they should be removed. The excavation should be performed with the use of dewatering techniques such as wellpoints and pumping from sumps. No more than 4-inches of water should be allowed in the bottom of the excavation. The sides of the excavation should be sloped or shored to protect workers as required by OSHA. During the excavation operations, any deleterious materials encountered should be hauled off site. No fill should be stockpiled adjacent to the excavation. During the replacement process a maximum of 2 feet of fill may be initially replaced without compaction. After which, fill should be compacted in lifts not exceeding 12 inches and should be compacted to 95 percent of the Modified Proctor (ASTM D-1557). Backfill material should be at or below the optimum moisture content for compaction and have no more than 5 percent material finer than a No. 200 sieve. Prior to performing the site development, the proposed earthwork plans and specifications should be provided to MACTEC for review.

7.4 BEARING CAPACITY

Based upon the satisfactory completion of the recommended site preparation, shallow foundations should be designed for a net maximum allowable bearing pressure of 2,500 pounds per square foot (psf). The foundation and floor slabs should bear on properly placed and compacted cohesionless (sand) fill.

All footings should be embedded so that the bottom of the foundation is a minimum of 16 inches below the adjacent compacted grades on all sides. Strip or wall footings should be a minimum of 18 inches wide and pad or column footings should be a minimum of 24 inches wide. These minimum footing sizes should be used regardless of whether or not the foundation loads and allowable bearing pressures dictate a smaller size. These minimum footing sizes tend to provide adequate load bearing area to develop overall bearing capacity and account for minor variations in the bearing materials.

7.5 SETTLEMENT

The settlement of shallow foundations supported on sandy-type soils should occur rapidly. Thus, after removal of the highly compressible material, the majority of expected settlements should occur during construction as dead loads are imposed at the footing locations. Provided that the recommended subgrade preparation operations are properly performed, the total settlements of isolated columns and wall footings should not exceed one inch, with differential settlements on the

Brandon Hyatt Hotel Brandon, Florida MACTEC Project 6513-08-0588

order of 60 percent of the total settlements. Differential settlements of these magnitudes are usually considered tolerable for the anticipated construction. The tolerance of the proposed structures to the predicted total and differential settlements should, however, be confirmed by the Structural Engineer.

7.6 SUITABILITY OF EXISTING SOILS AS FILL

Fill should generally consist of dry fine sand with less than 5 percent passing the No. 200 sieve, free of rubble, organics, clay, debris and other unsuitable material. The majority of the soils encountered in the upper regions of the borings do not meet this criterion therefore would not meet criteria for suitable structural fill.

8.0 BASIS FOR RECOMMENDATIONS

Our professional services have been performed, our findings obtained and our preliminary recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This company is not responsible for the conclusions, opinions or recommendations made by others based on the data presented in this report.

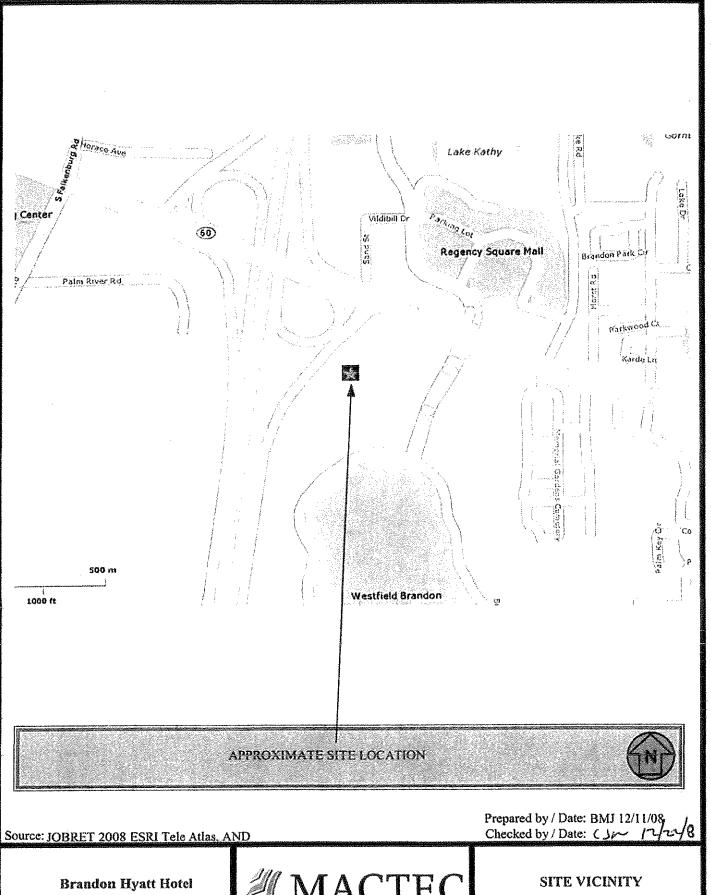
The analysis and recommendations submitted in this report are based upon the data obtained from the soil borings performed at the locations indicated. Regardless of the thoroughness of a geotechnical exploration, there is always a possibility that conditions between borings will be different from those at specific boring locations and that conditions will not be as anticipated by the designers or contractors. In addition, the construction process itself may alter soil conditions.

If any subsoil variations become evident during the course of this project, a re-evaluation of the recommendations contained in this report will be necessary after we have had an opportunity to observe the characteristics of the conditions encountered. The applicability of the report should also be reviewed in the event significant changes occur in the design, nature or location of the proposed structure.

The recommendations provided in this report are based in part on project information provided to us and they only apply to the specific project and the site discussed in this report. If the project information is incorrect, or if additional information is available, the correct or additional information should be conveyed to us for review. Our recommendations may then be modified, if necessary. Experienced geotechnical personnel should observe and document the construction procedures used and the conditions encountered. Unanticipated conditions and inadequate procedures should be reported to the design team. We recommend that the owner retain MACTEC to provide these services based upon our familiarity with the project, the subsurface conditions and the intent of the recommendations and design.





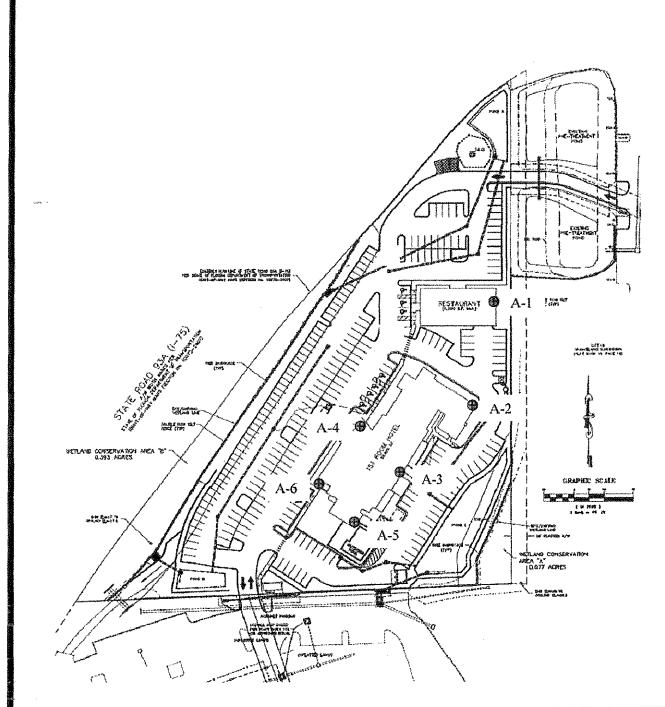


Hillsborough County, Florida



6513-08-0588

Figure: 1



APPROXIMATE SOIL TEST BORING LOCATIONS



Source: Love Investment Company, 4/16/08

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Brandon Hyatt Hotel Hillsborough County, Florida

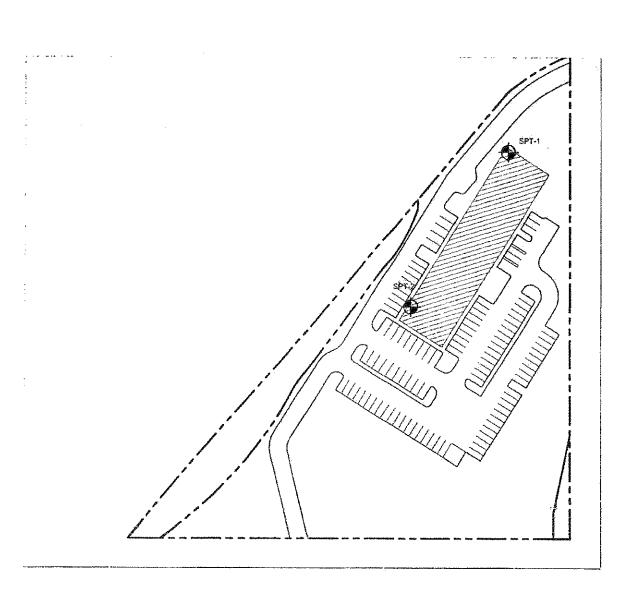


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FIELD EXPLORATION PLAN

6513-08-0588

Figure: 2A



Brandon Hotel Site Development Brandon, Florida

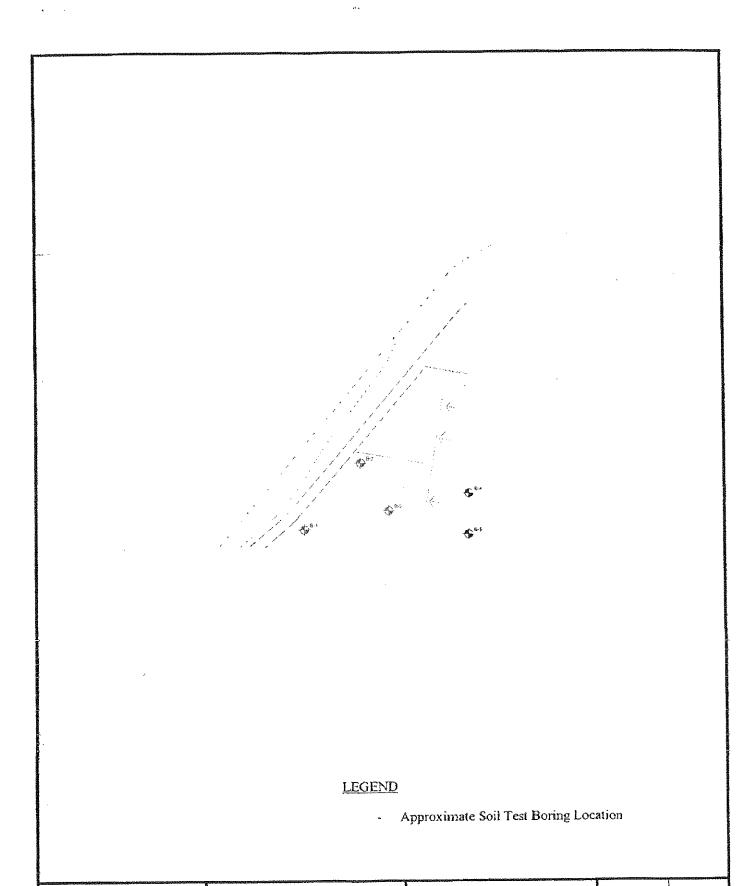


FIELD EXPLORATION PLAN Project 6513-06-0422

Figure 2.8

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Brandon Hotel Site Development Brandon, Florida

MACTEC

FIELD EXPLORATION PLAN

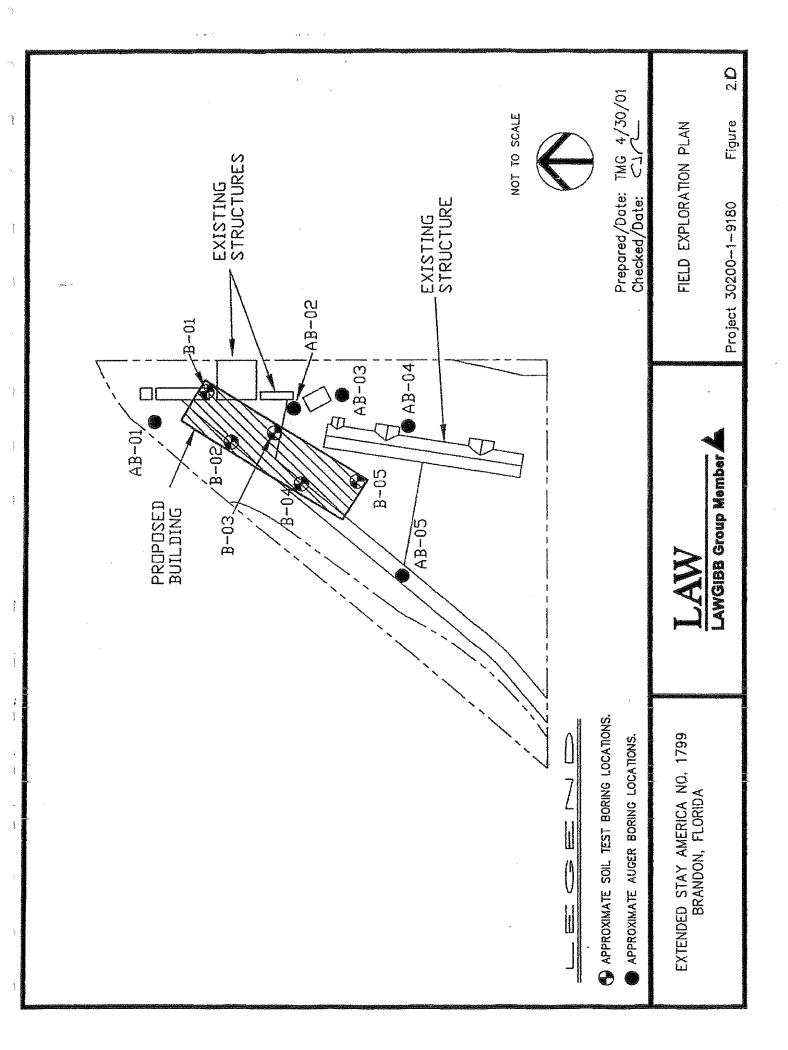
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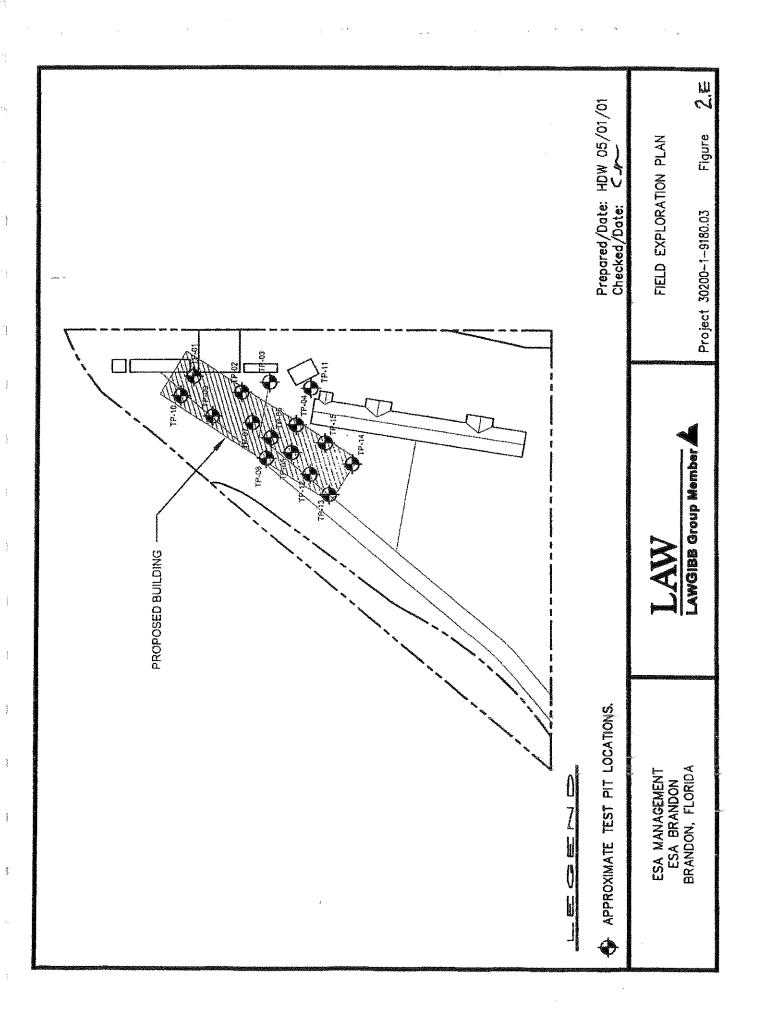
Project 6513-04-0349 Figure 2, **C**

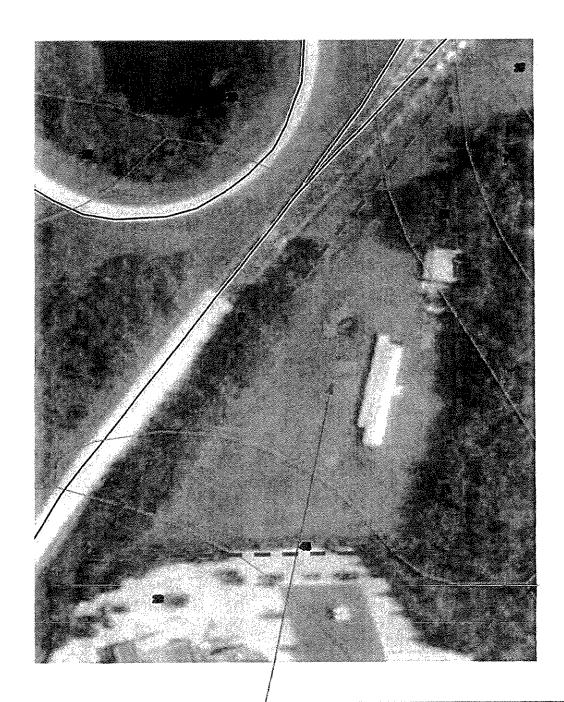
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APPROXIMATE SITE LOCATION



Source: USDA "Soil Survey of Hillsborough County, Florida"

Brandon Hyatt Hotel Hillsborough County, Florida

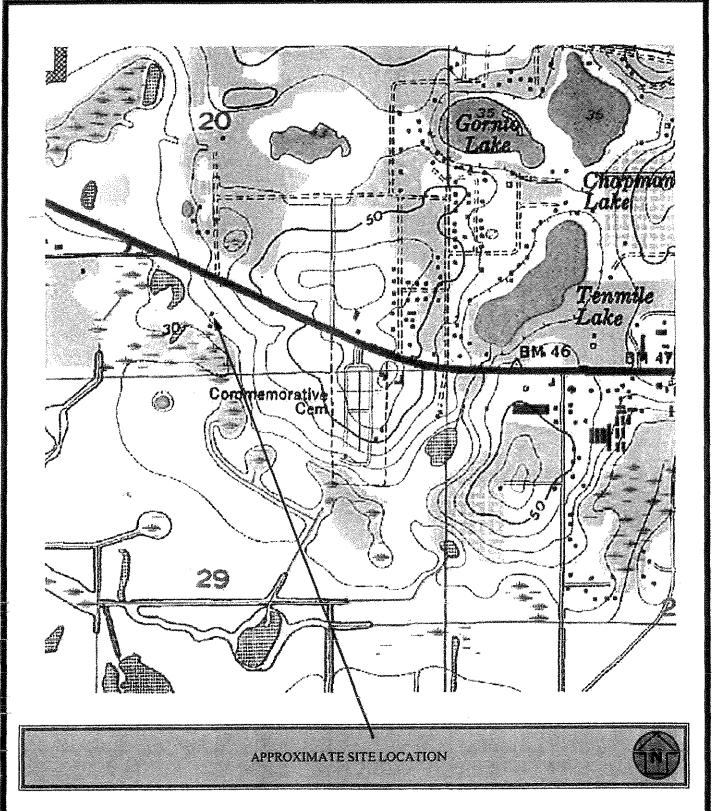


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USDA VICINITY MAP

6513-08-0588

Figure: 3



Source: USGS Quadrangle Map, Brandon, Florida 1987

Prepared by / Date: BMJ 12/11/08

Checked by / Date:

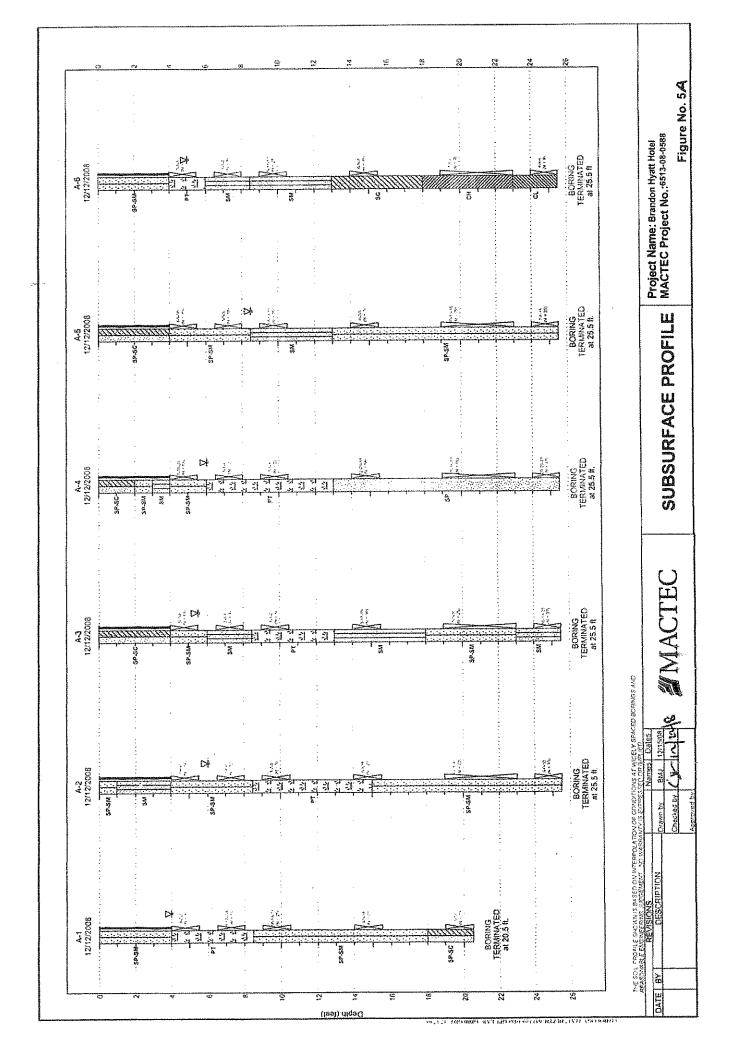
Brandon Hyatt Hotel Hillsborough County, Florida

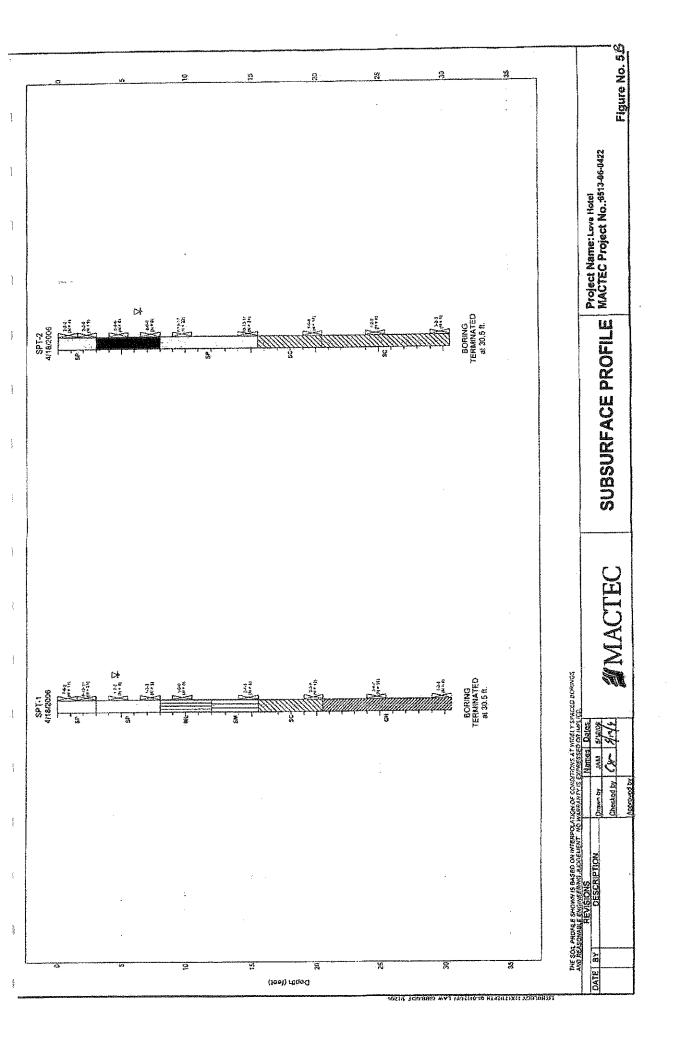


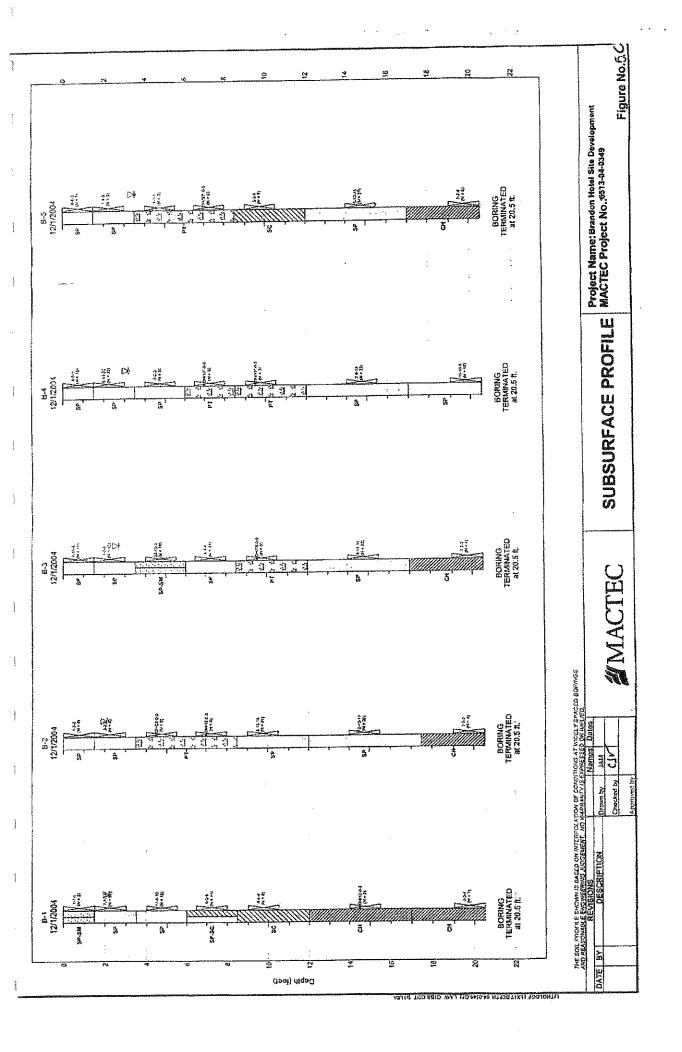
USGS VICINITY MAP

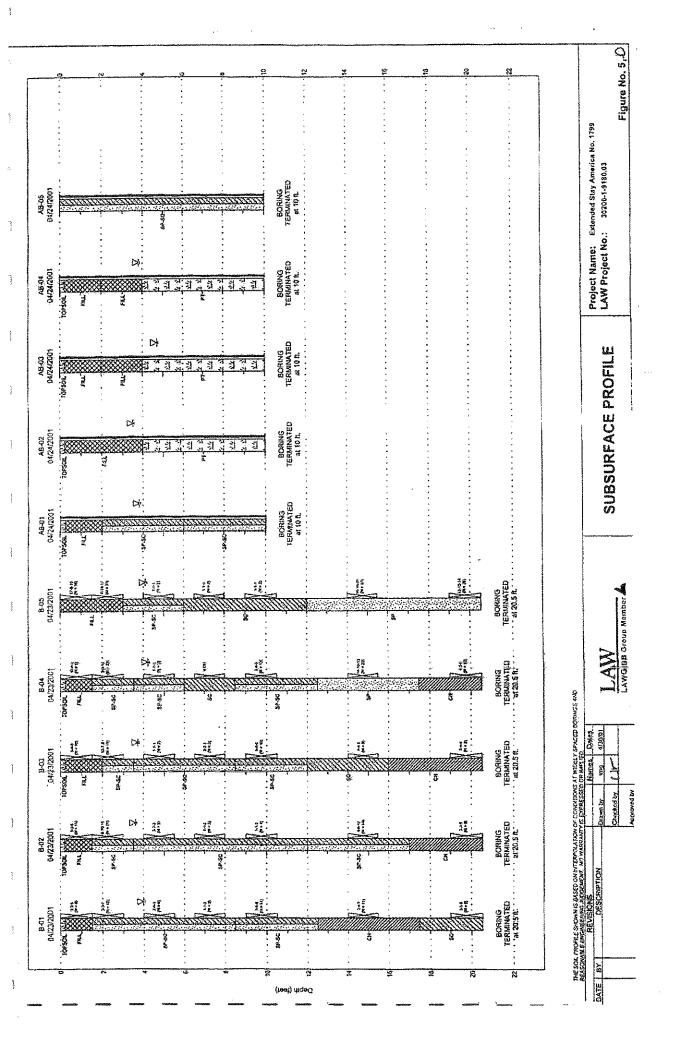
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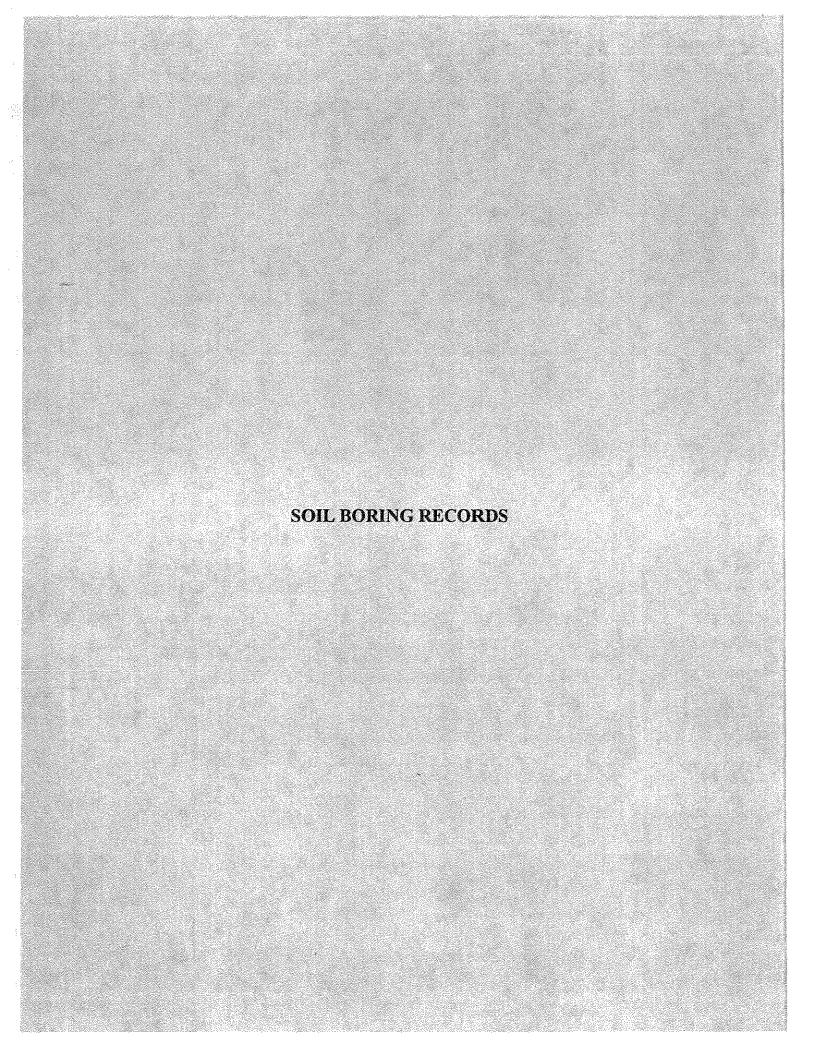
Figure: 4

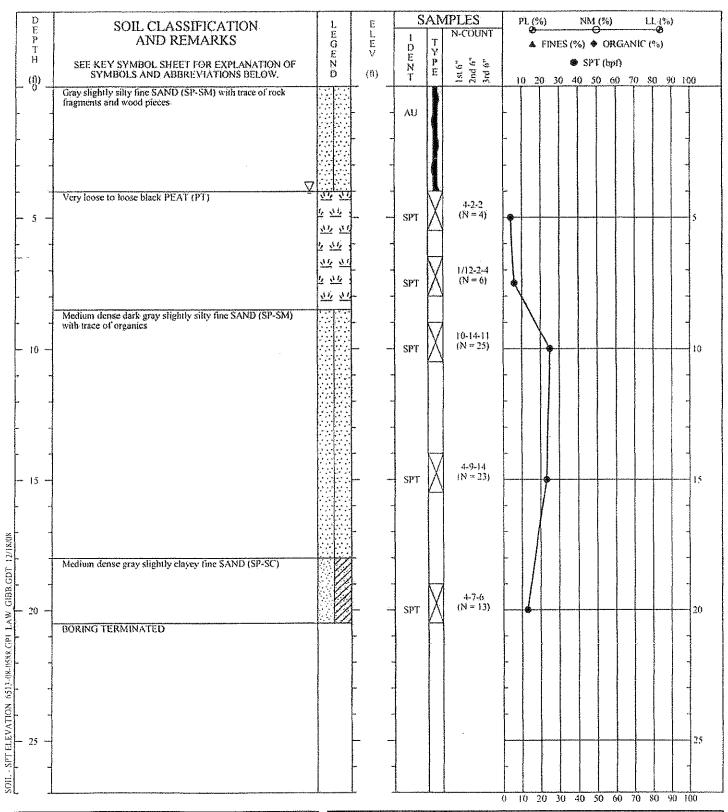












DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

Hammer

Auger Boring, ASTM D-1452

METHOD: REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel

LOCATION: Brandon, FL

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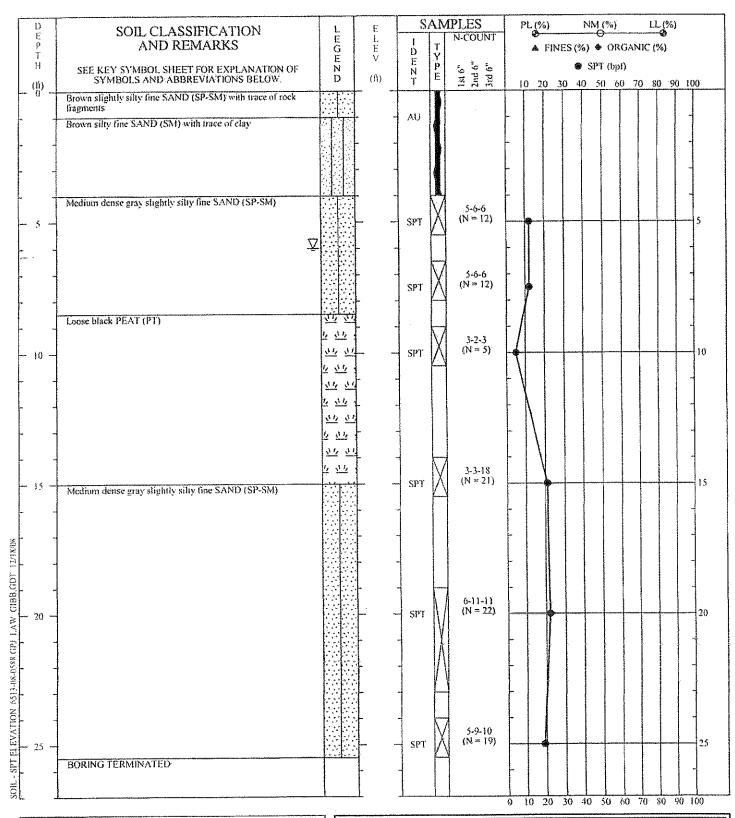
PROJ. NO.:

December 12, 2008 6513-08-0588

BORING NO.: A-1

PAGE 1 OF 1 CHECKED BY:





DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

METHOD: Hammer

HOLE DIA .: Auger Boring, ASTM D-1452

REMARKS:

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SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel

LOCATION: Brandon, FL

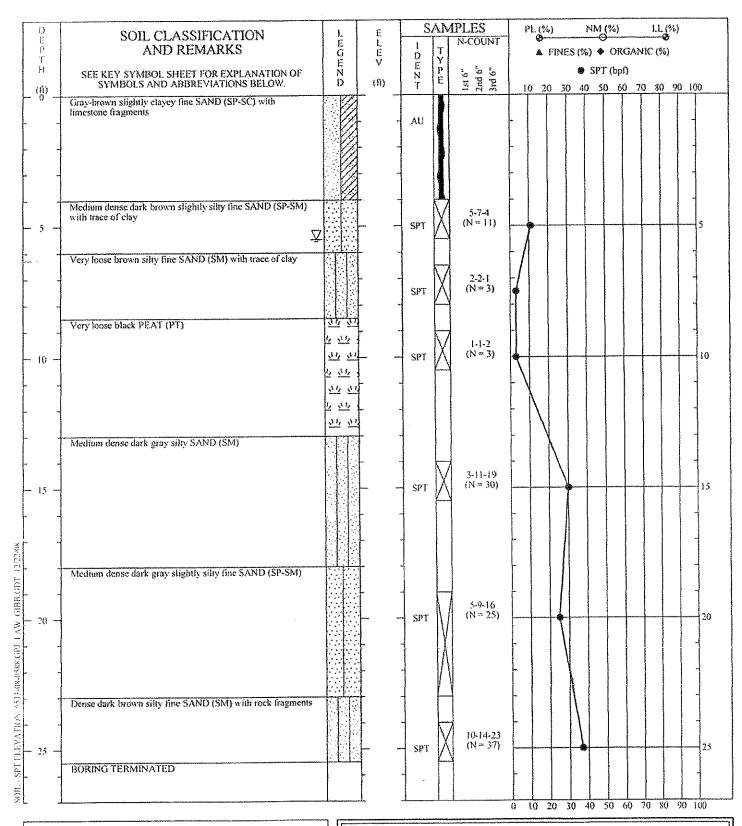
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6513-08-0588

BORING NO.: A-2 PAGE 1 OF 1

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DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

METHOD: Hammer

Auger Boring, ASTM D-1452 HOLE DIA.:

REMARKS:

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SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel

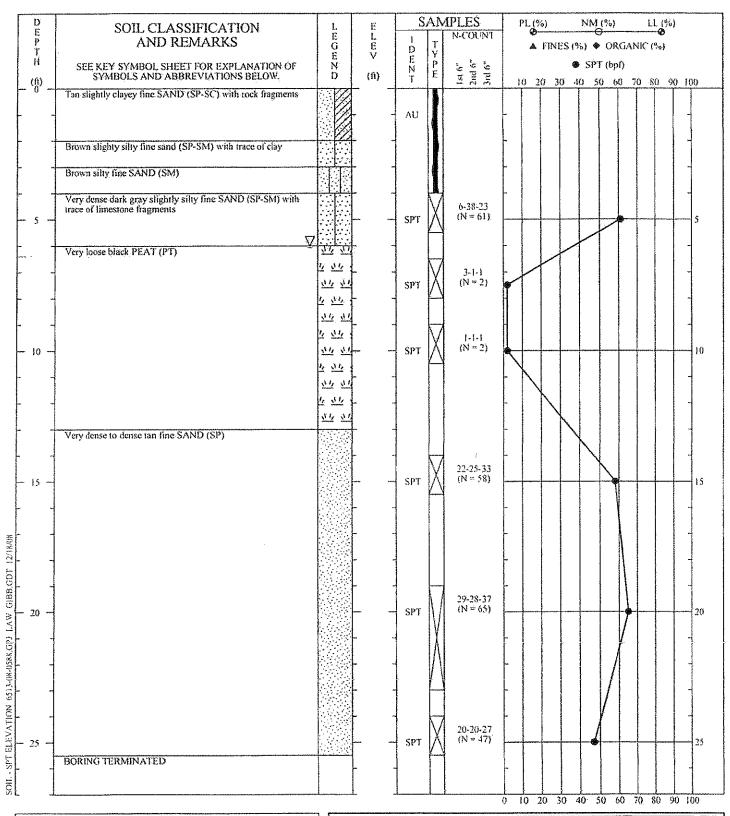
LOCATION: Brandon, FL

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PAGE 1 OF 1 CHECKED BY: 6513-08-0588

BORING NO.: A-3





DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

METHOD: Hammer

HOLE DIA.: Auger Boring, ASTM D-1452

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel LOCATION: Brandon, FL

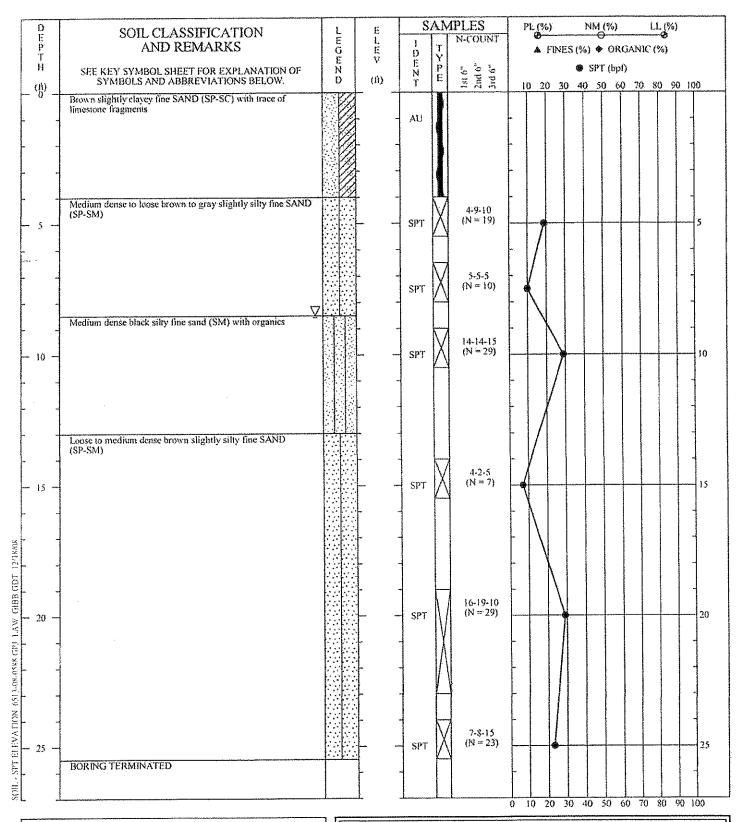
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PAGE 1 OF J

BORING NO.: A-4





DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

METHOD: Hammer

HOLE DIA.: Auger Boring, ASTM D-1452

REMARKS:

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SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel LOCATION: Brandon, FL

DRILLED: December 12, 2008

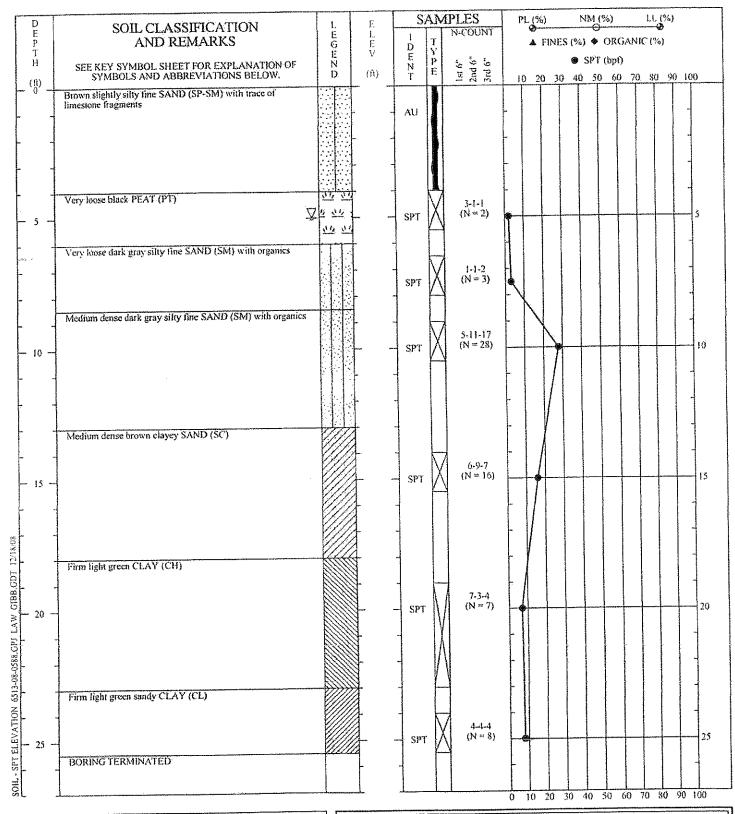
6513-08-0588

PROJ. NO.:

BORING NO.: A-5

PAGE 1 OF CHECKED BY: (')





DRILLER: Mark Murray

EQUIPMENT: Power Drill Rig with Hollow Stem Auger and Cathead

METHOD: Hammer

HOLE DIA.: Auger Boring, ASTM D-1452

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hyatt Hotel

LOCATION: Brandon, FL

PROJ. NO.:

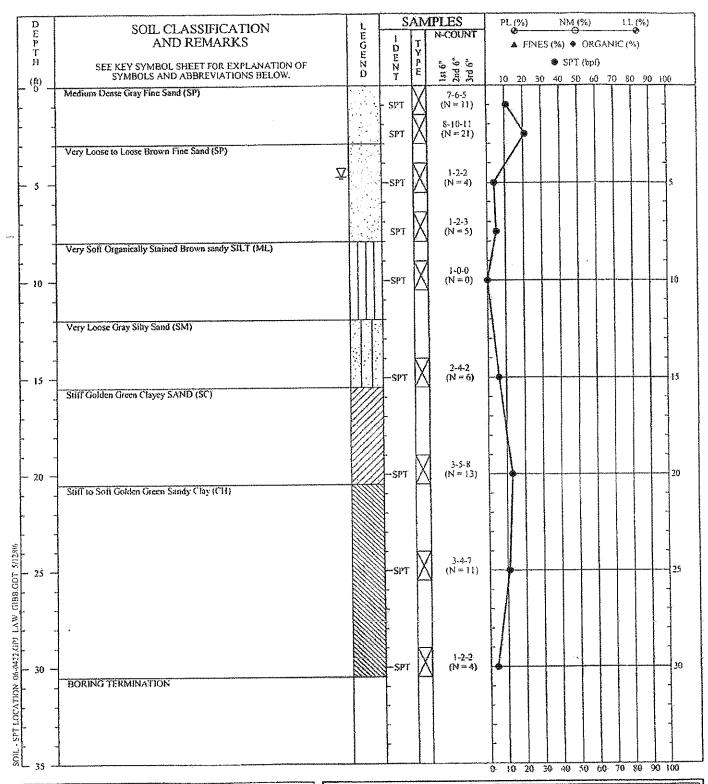
DRILLED: December 12, 2008

PAGE 1 QF 1

BORING NO.: A-6

6513-08-0588 CHECKED BY:





DRILLER: K. Jakacky
EQUIPMENT: CME Power Drill Rig
METHOD: Rotary Wash with Bennonite "Mud". ASTM D-1586
HOLE DIA.: 2 15'16 inches
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

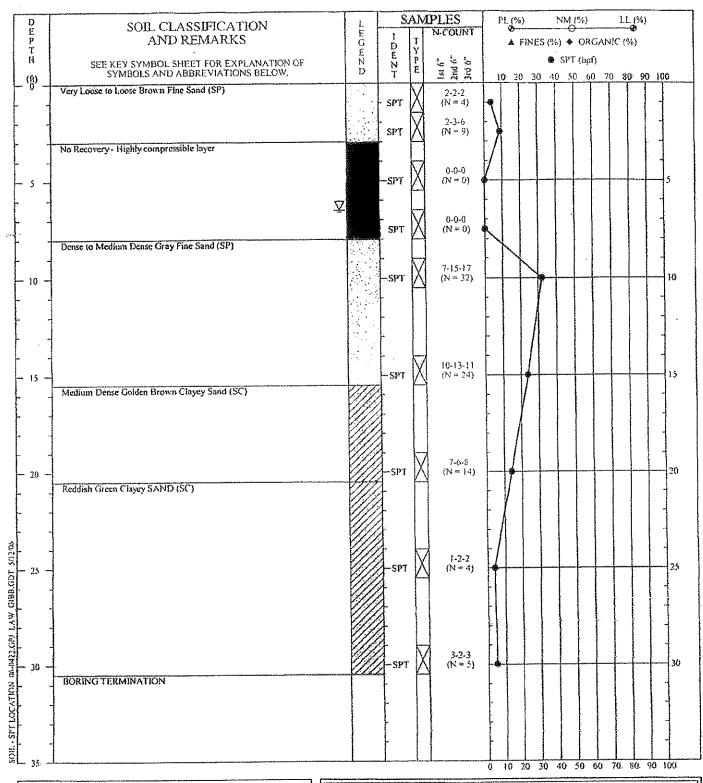
SOIL TEST BORING RECORD

PROJECT: Love Hotel LOCATION: Brandon, FL

DRILLED: April 18, 2006 **PROJ. NO.:** 6513-06-0422

PAGE 1 OF 1 CHECKED BY: CH





DRILLER: A K. Jakacky
EQUIPMENT: CME Power Drill Rig
METHOD: Rotary Wash with Bentonite "Mud". ASTM D-1586
HOLE DIA: 2 15/16 inches
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

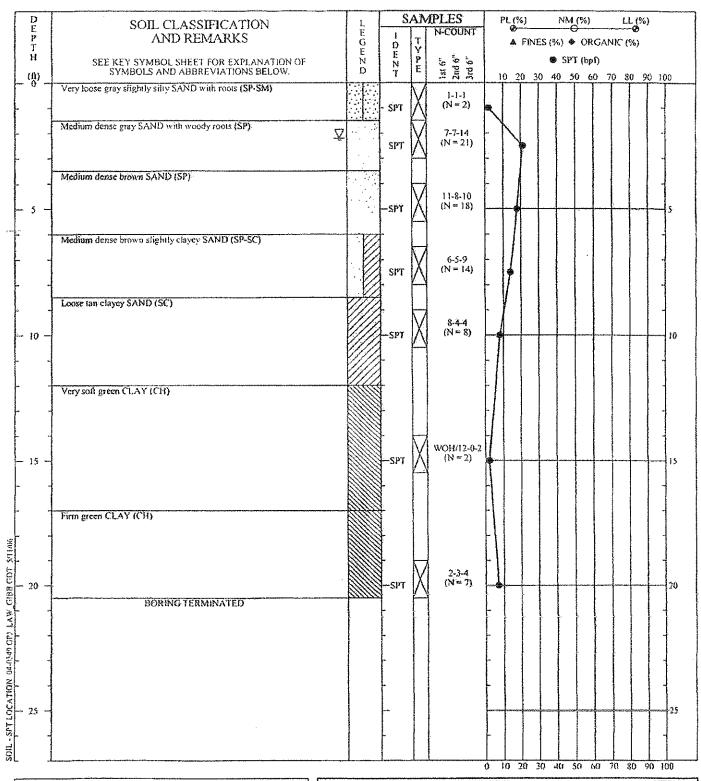
SOIL TEST BORING RECORD

PROJECT: Love Hotel LOCATION: Brandon, FL

DRILLED: April 18, 2006 **PROJ. NO.:** 6513-06-0422

PAGE 1 OF 1 CHECKED BY:





DRILLER:

D. Teslicko

EQUIPMENT: CME Power Drill Rig

METHOD:

Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA.: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hotel Site Development

LOCATION: Brandon, FL

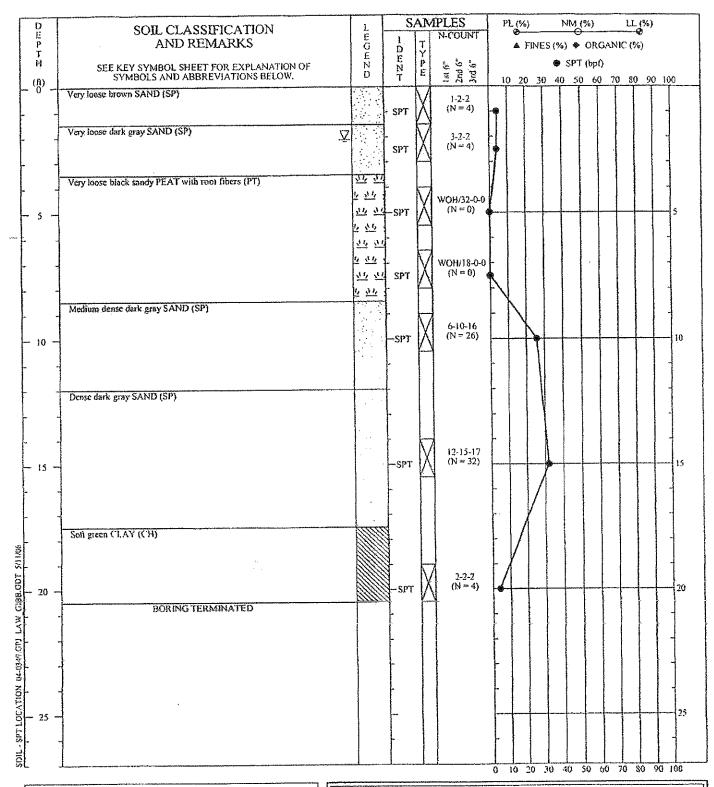
BORING NO.: B-1

DRILLED: December 1, 2004

PROJ. NO.: 6513-04-0349

PAGE 1 OF 1 CHECKED BY:





DRILLER: D. Teslicko
EQUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA.; 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hotel Site Development

LOCATION: Brandon, FL

PROJ. NO.:

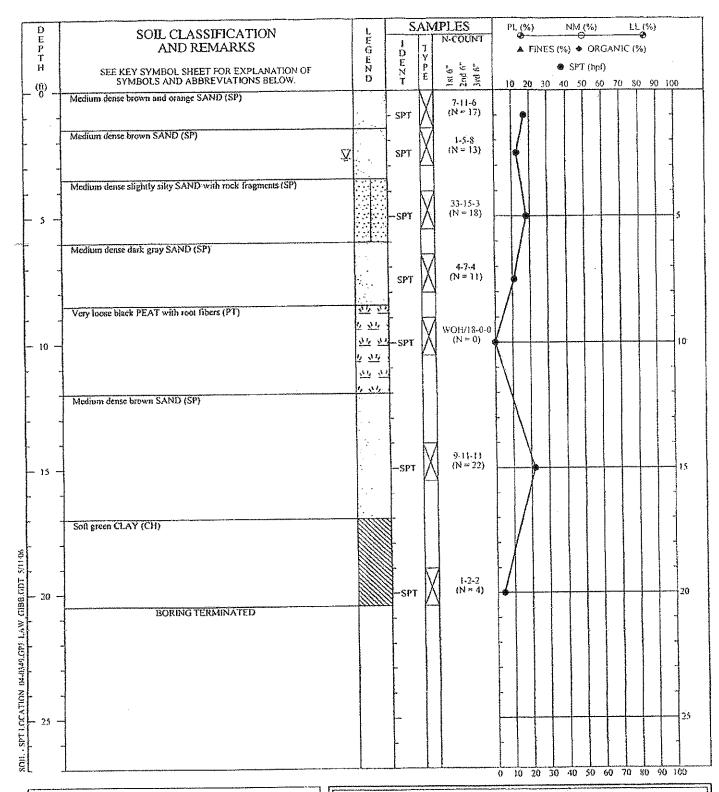
DRILLED: December 1, 2004

BORING NO.: B-2 PAGE 1 OF 1

6513-04-0349

CHECKED BY: C





DRILLER:

D. Teslicko

EQUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bentonite "Mud", ASTM D-1586 2 15/16 inches

HOLE DIA.

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hotel Site Development

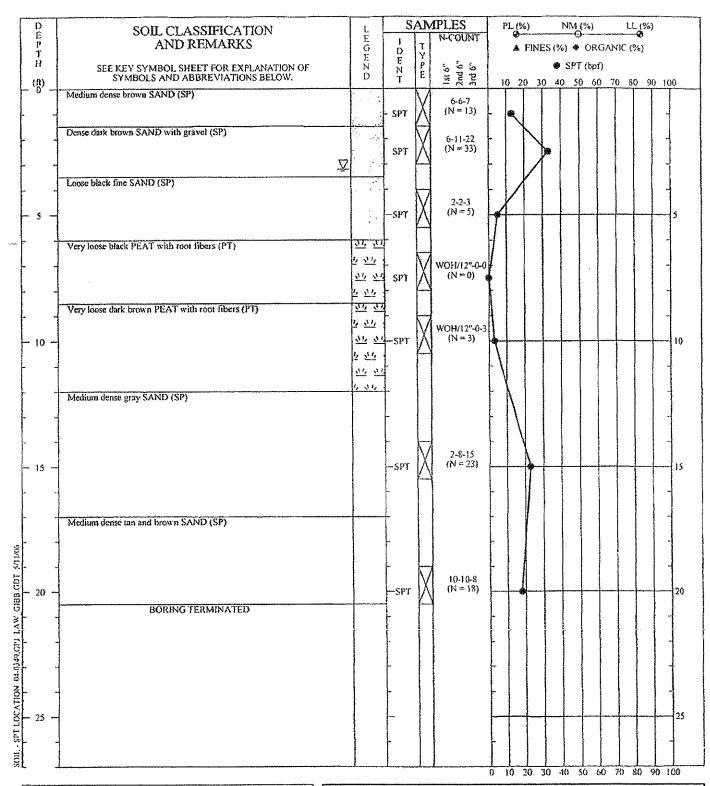
LOCATION: Brandon, FL

DRILLED: December 1, 2004 PROJ. NO.: 6513-04-0349

BORING NO.: B-3 PAGE I OF I

CHECKED BY: Cr





DRILLER: D. Testicko
EQUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bemorite "Mad". ASTM D-1586

HOLE DIA: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT: Brandon Hotel Site Development

LOCATION: Brandon, FL

PROJ. NO.:

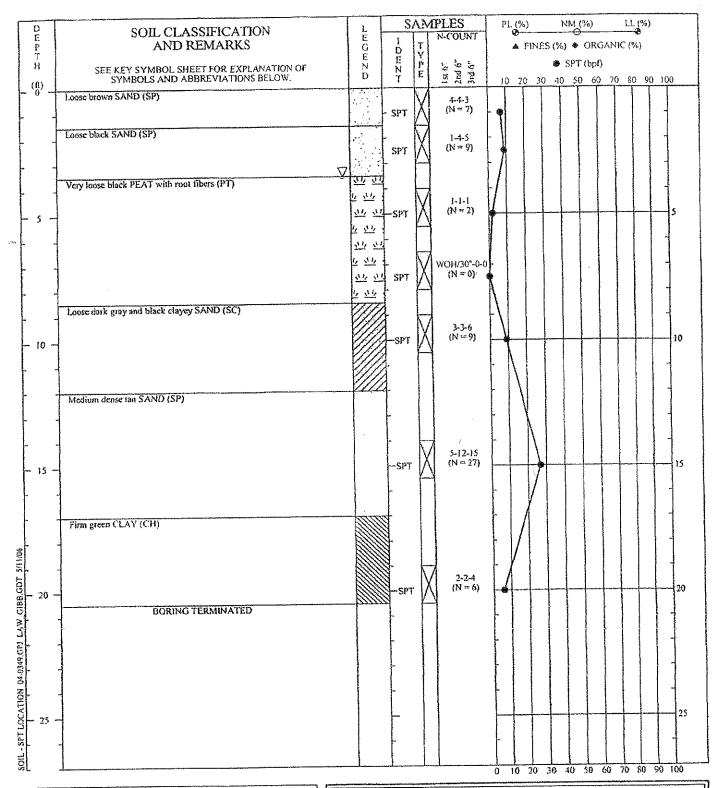
DRILLED: December 1, 2004

6513-04-0349

PAGE 1 OF 1 CHECKED BY: CJ

BORING NO.: B-4





DRILLER:

EOUIPMENT: CME Power Drill Rig

Rotary Wash with Bentonite "Mud", ASTM D-1586 METHOD:

HOLE DIA: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

SOIL TEST BORING RECORD

PROJECT:

Brandon Hotel Site Development

LOCATION:

DRILLED:

PROJ. NO.:

Brandon, FL

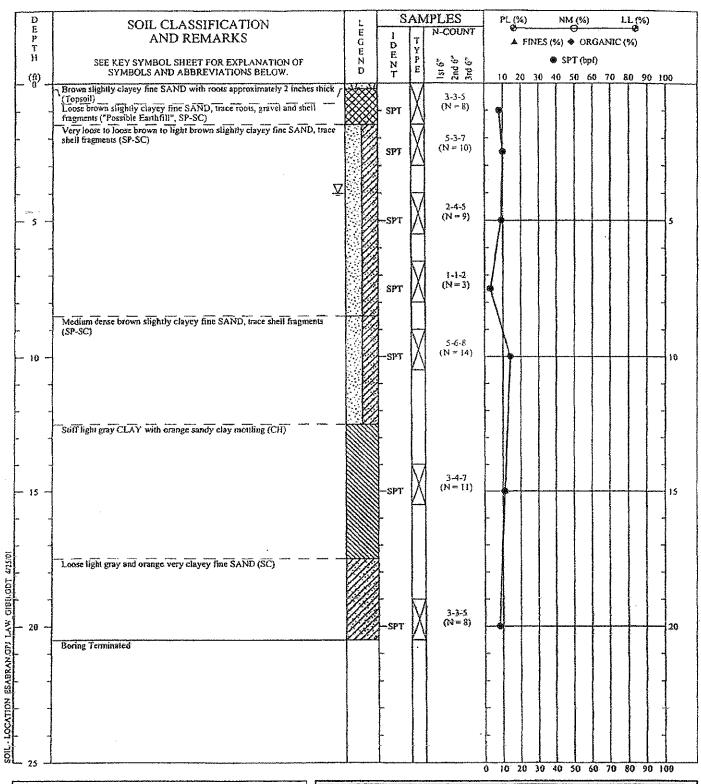
December 1, 2004

6513-04-0349

BORING NO.: B-5

PAGE 1 OF 1
CHECKED BY:





DRILLER: EQUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA.: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: Extended Stay America No. 1799

LOCATION: Brandon, Florida

BORING NO.: B-01

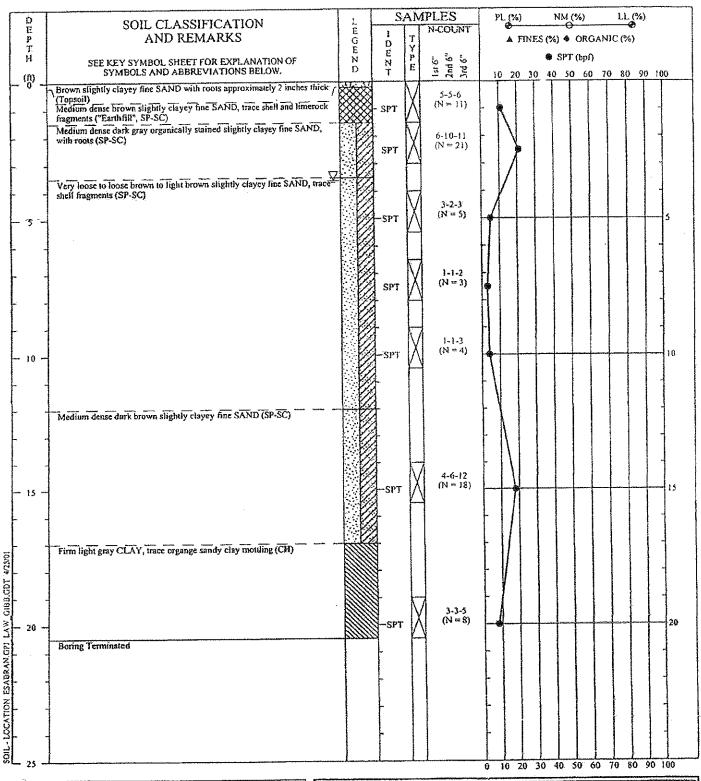
DRILLED: April 23, 2001 PROJ. NO.:

PAGE 1 OF 1 CHECKED BY: C

30200-1-9180.03

LAWGIBB Group Member





DRILLER: EQUIPMENT: CME Power Drill Rig METHOD:

Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA.: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL

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PROJECT: Extended Stay America No. 1799

LOCATION: Brandon, Florida

DRILLED:

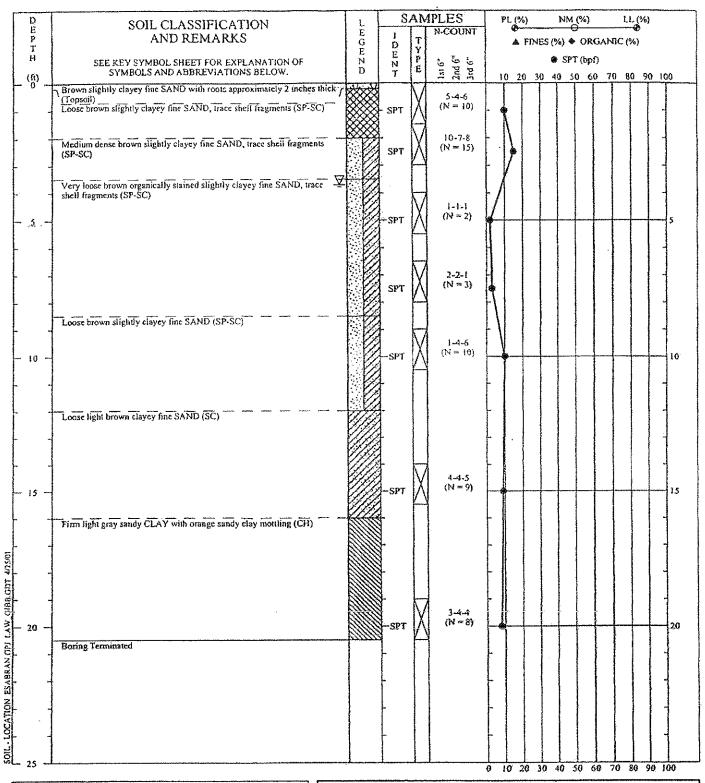
BORING NO.: B-02 PAGE 1 OF 1

April 23, 2001 PROJ. NO.: 30200-1-9180.03

CHECKED BY: ()







DRILLER:

EQUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA.: 2 15/16 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION THIS RECURD IS A REASONABLE IN ERFRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: Extended Stay America No. 1799

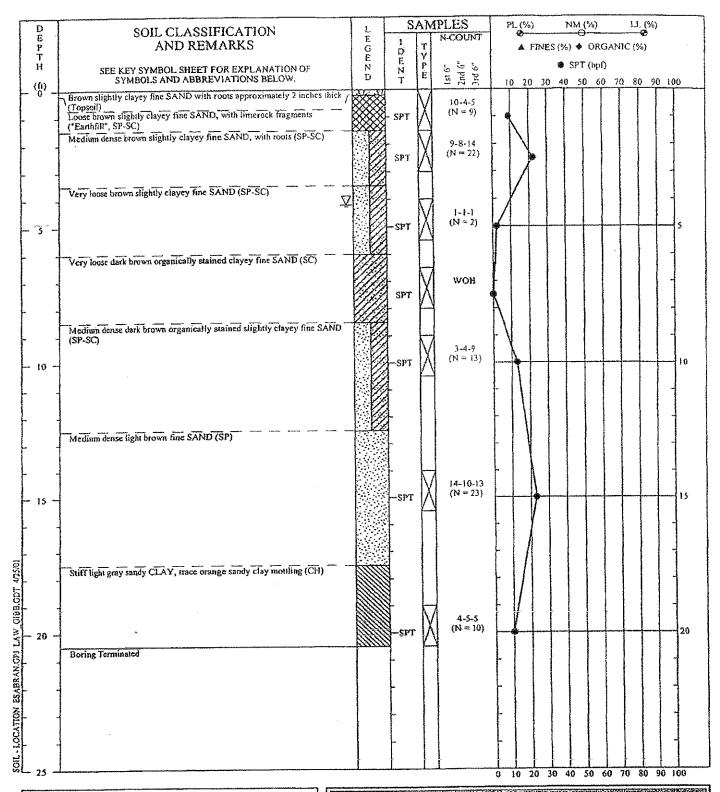
LOCATION: Brandon, Florida

DRILLED: April 23, 2001 **BORING NO.: B-03** PAGE I OF I

CHECKED BY: (3) PROJ. NO.: 30200-1-9180.03

LAWGIBB Group Member





DRILLER:

D. Testicko

EQUIPMENT: CME Power Drift Rig

METHOD:

Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA .: 2 15/16 mches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS RETWEEN STRATA MAY BE GRADUAL.



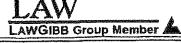
PROJECT: Extended Stay America No. 1799

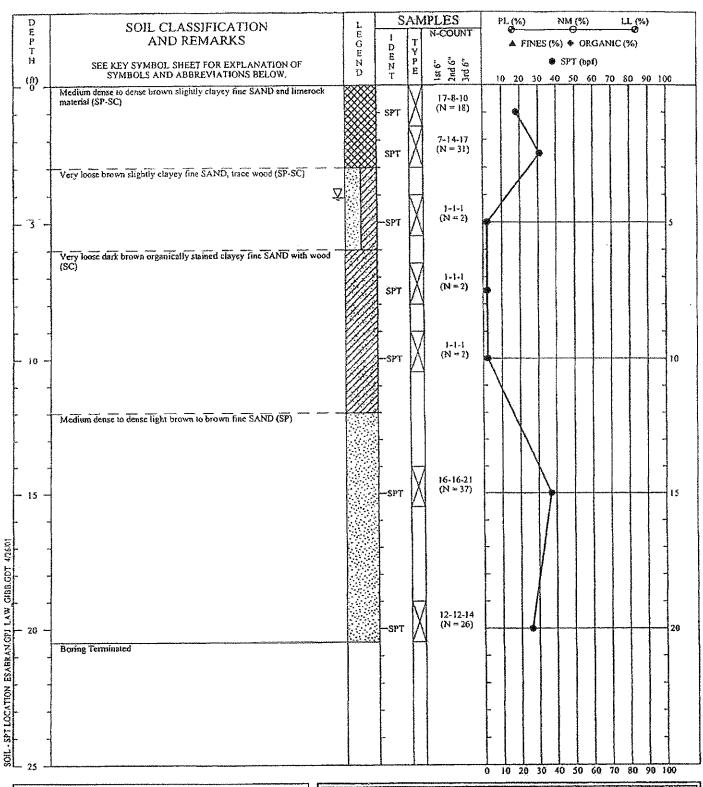
LOCATION: Brandon, Florida

BORING NO.: B-04

DRILLED: April 23, 2001 **PROJ. NO.:** 30200-1-9180.03

PAGE 1 OF 1 CHECKED BY:





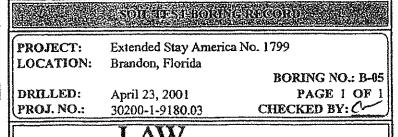
DRILLER: D. Teslicko
EOUIPMENT: CME Power Drill Rig

METHOD: Rotary Wash with Bentonite "Mud", ASTM D-1586

HOLE DIA: 2 15/16 inches

REMARKS:

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LAWGIBB Group Member A

SAMPLES PL (%) NM (%) LL (%) SOIL CLASSIFICATION LEGEND N-COUNT AND REMARKS TYPE ▲ FINES (%) DENT SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW. 158 6° 259 6° 378 6° ORGANICS (%) 10 20 30 40 50 60 70 80 90 100 Brown slightly clayey fine SAND with roots approximately 2 inches thick (Topsoil)
Brown slightly clayey fine SAND, trace shell Tragments ("Earthfill", SP-SC) ΑÜ Brown slightly clayey fine SAND (SP-SC) Light brown slightly clayey fine SAND (SP-SC) 10 10 Boring Terminated 15 15 4725/01 CIBB.CDT * `` ESABRANGP) 20 NOTER BORING LOCATION 20 30 40 50 60 70 80 90 100

DRILLER: D. Teslicko
EQUIPMENT: 3-inch Diameter Bucket Auger
METHOD: Solid Stem Auger Boring, ASTM D-1452
HOLE DIA.: 3 inches
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

AUGERBORESERE ORD SEE

PROJECT: Extended Stay America No. 1799

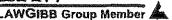
LOCATION: Brandon, Florida

DRILLED: April 24, 2001

PAGE 1 OF 1

PROJ. NO.: 30200-1-9180.03 CHECKED BY: C

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D	SOIL CLASSIFICATION	L		A۱	APLES N-COUNT	I	۲L (%	6)	,	NM	(%)		LI	L (%)		
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Γ "	Brown slightly clayey fine SAND with roots approximately 2 inches thick (Topsoil) Brown slightly clayey fine SAND, trace roots ("Earthfill", SP-SC)															
-	brown suggetty clayey time SAND, trace roots (Eartiflite, Sr-SC)		- AU			-									4	
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-	Dark brown peat, with root fibers (PT)	XXXX	•			-										
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DRILLER: D. Teslicko

EQUIPMENT: 3-inch Diameter Bucket Auger

METHOD: Solid Stem Auger Boring, ASTM D-1452

HOLE DIA .: 3 inches

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

AUGERBORINGRECORD

PROJECT: Extended Stay America No. 1799

LOCATION: Brandon, Florida

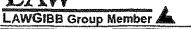
PROJ. NO.:

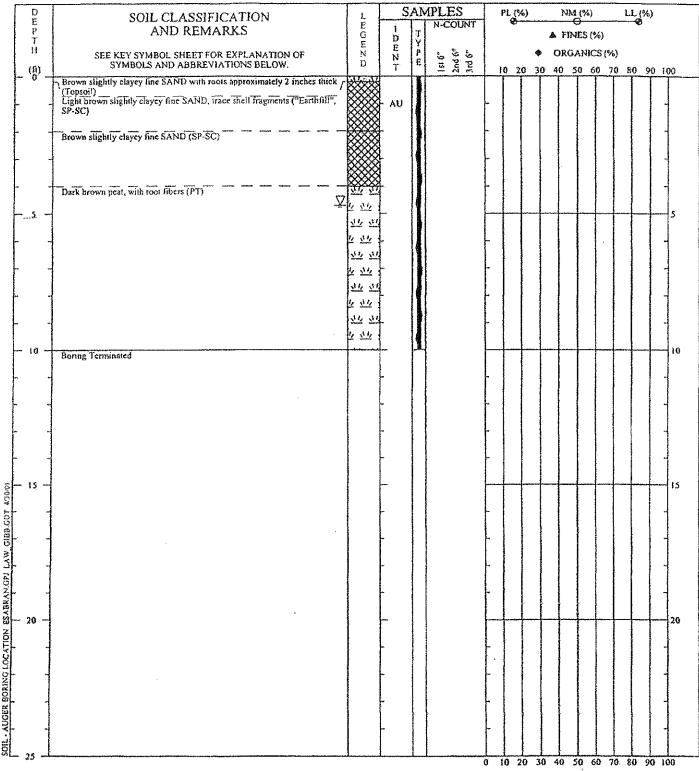
DRILLED: April 24, 2001

BORING NO.: AB-02

PAGE 1 OF J CHECKED BY: C

30200-1-9180:03 Y **AXX**7





DRILLER: D. Teslicko
EQUIPMENT: 3-inch Diameter Bucket Auger
METHOD: Solid Stem Auger Boring, ASTM D-1452
HOLE DIA.: 3 inches
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

A GEREBORING RECORD.

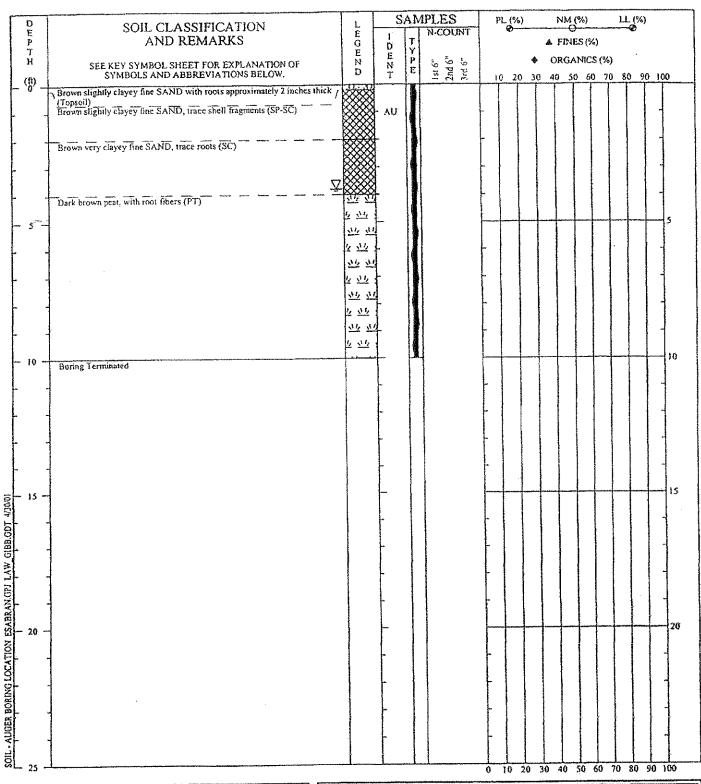
PROJECT: Extended Stay America No. 1799

LOCATION: Brandon, Florida

BORING NO.: AB-03 ril 24, 2001 PAGE 1 OF 1

DRILLED: April 24, 2001 PAGE 1 OF 1
PROJ. NO.: 30200-1-9180.03 CHECKED BY: CF





DRILLER: D. Teslicko

EQUIPMENT: 3-inch Diameter Bucket Auger

Solid Stem Auger Boring, ASTM D-1452 METHOD:

HOLE DIA.:

REMARKS:

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TRANSITIONS RETWEEN STRATA MAY BE GRADUAL.

MUGLE BORING RECORD

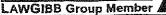
Extended Stay America No. 1799 PROJECT:

LOCATION: Brandon, Florida

DRILLED: April 24, 2001 BORING NO.: AB-04

PROJ. NO.: 30200-1-9180.03 PAGE 1 OF 1 CHECKED BY:







SAMPLES PL (%) NM (%) LL (%) SOIL CLASSIFICATION LEGEND N-COUNT AND REMARKS LOENT TYPE A FINES (%) T SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW. 151 6" 2nd 6" 3rd 6" ORGANICS (%) 10 20 30 40 50 60 70 80 90 100 Brown slightly clayey fine SAND (SP-SC) 10 Bonng Terminated 15 1.5 ESABRAN.GPJ 20 20 20

DRILLER: D. Teslicko

EQUIPMENT: 3-inch Diameter Bucket Auger

METHOD: Solid Stem Auger Boring, ASTM D-1452

HOLE DIA.: 3 inches

REMARKS:

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EAST AUGERBORING RECORDS

PROJECT: Extended Stay America No. 1799

LOCATION: Brandon, Florida

BORING NO.: AB-05

DRILLED: April 24, 2001 PAGE 1 OF 1
PROJ. NO.: 30200-1-9180.03 CHECKED BY:

LAW

LAWGIBB Group Member

, a	SOIL CLASSIFICATION	L	E	SA	MPLE	T
P	SOIL CLASSIFICATION AND REMARKS	LE GEN	E L E V	DENT	Dry Donalty (per)	Moisture Content %
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- '0' -	2 inches of Topsoil Brown slightly sitry fine SAND with small and large 1001s, trace debris ("Earthfill", SP-SM)			8		
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	Brown slightly sitty fine SAND (SP-SM)	***				
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DRILLER: Complete Development EQUIPMENT: Trackhoe HOLE DIM.: REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-01

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

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'0' - 2 in	nches of Topsoil ncrock base material rk brown organically stained slightly silty fine SAND ("Earthfill", SP-SM)	W	- 0.0 -		<u> </u>		
Dar	rk brown organically stained slightly silty fine SAND ("Earthfill", SP-SM)						
Bro	own slightly sitty fine SAND (SP-SM)	XXX	-		KS8		
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DRILLER: Complete Development EQUIPMENT: Trackhoe HOLE DIM.: REMARKS:

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THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-02

EXCAVATED: May 18, 2001 30200-1-9180 PROJ. NO.:

D E	SOIL CLASSIFICATION	Į.	Ę		T	IPLE	S
r T	SOIL CLASSIFICATION AND REMARKS SEE KEY SYMBOL SHEET FOR EXPLANATION OF	LEGEN	ELEV	E N T	T Y P E	Dry Density (pcf)	Moisture
(i)	Symbols and abbreviations below.	Ď	(ft) - 0.0 -	T	Ē	à	X
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LD	bark brown organically stained slightly silty fine SAND with large roots, some debris and seams of dark brown peat		9 .				
(5	SP-SM)						
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DRILLER: Complete Development
EQUIPMENT: Trackhoe
HOLE DIM.:
REMARKS:

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

EXCAVATED: May 18, 2001

PROJ. NO.: 30200-1-9180

TEST PIT NO.: TP-03

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(P)	2 inches of Topsoil Limerock base material	- 177, 17	0.0		₹₹ <u></u>	_
-	Gray to brown slightly silty fine SAND ("Earthfill", SP-SM)	🍇			\$ \$\$	
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	Brown to light brown fine SAND (SP)				} }}	
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DRILLER: Complete Development EQUIPMENT: Trackhoe HOLE DIM.:

HOLE DIM.: REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS RETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-04

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

PAGE I OF 1

SOIL CLASSIFICATION	1 1	E L	Sz	AMPLE	7
AND REMARKS	-1 E C E X	E V	D D	T È	22
SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW.	D	(ft) - 0.0	D E N T	Dry Density (pc)	Moisture
2 inches of Topsoil Limerock base material 4 inches thick		۷.۷	ţ	; <u>;</u> ;	
Gray to brown slightly siity fine SAND (SP-SM)			ĺ	}}}	
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Dark brown PEAT with small and large roots at 3 feet (PT)	<u></u>	- "		} }}	
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Brown fine SAND (SP)		-			
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DRILLER: Complete Development EQUIPMENT: Trackbox HOLE DIM.:

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION:

Brandon, Florida

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

TEST PIT NO.: TP-05

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(f) + 3	inches of Topsoil	<u> </u>	- 0,0 -		22		-	
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 DRILLER: EQUIPMENT:	Complete Development Trackhoe
HOLE DIM.: REMARKS:	

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION, SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER, INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

EXCAVATED: May 18, 2001

PROJ. NO.: 30200-1-9180

TEST PIT NO.: TP-06

PAGE 1 OF 1

00-1-9180 PAG

D E	SOIL CLASSIFICATION		E	S	AMPLE	S
H (fi)	AND REMARKS SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW.	EGEND	ELEV (A)	I D E N T	Dry Density (pc)	Moisture Content %
	Grass with limerock material 3 inches thick Gray to brown slightly silty fine SAND, with debris ("Earthfilt", SP-SM)		0.6			
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	Dark brown organically stained slighly silty fine SAND, with small roots (SP-SM)		-			
5	Dark brown to brown slighlty sitty fine SAND (SP-SM)		-S.O	BK		amely version destroys a south a second principal destroys destroys the second
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DRILLER: Complete Development EQUIPMENT: Trackhoc HOLE DIM.:

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ES LOCATION: Bra

ESA - Brandon Brandon, Florida

TEST PIT NO.: TP-07

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

PAGE I OF I

D E	SOIL CLASSIFICATION	L.	E	SA	MPLE	S
P. T.	AND REMARKS	E G E S	E L E V	DEX	Dry Density (pcf)	Moisture
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Dark brown PE	AT with small roots (PT)	F 48 1		Į.	{{}	
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DRILLER: Complete Development
EQUIPMENT: Trackhoe
HOLE DIM.:
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-08

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

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(8)	2 inches of Topsoil Limstock material with shell fragments	- 222	- 0.0 -	B	33	
-	Dark brown to gray slightly silty fine SAND with debris ("Earthfill", SP-SM)					
	Dark brown PEAT with small roots (PT)	24 24 25 25 25 25 25 25 25 25 25 25 25 25 25	-			- Lungalitares
-	Dark brown organically stained slightly silty fine SAND (SP-SM)					
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DRILLER: Complete Development
EQUIPMENT: Trackboe
HOLE DIM.:
REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-09

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

a	SOIL CLASSIFICATION	L	E	S	ΑN	IPLE	S
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DRILLER: Complete Development EQUIPMENT: Trackhoe HOLE DIM.:

REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-10

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

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E P T	SOIL CLASSIFICATION AND REMARKS	TEGEZ D	A L E	I D E N T	T Y P E	Dry Density (pef)	Moisture Content %
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	Brown to gray slightly silty fine SAND with debris ("Earthfill", SP-SM)						
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2004	Dark brown PEAT with small and large roots (PT)	4 34					
	Dark brown organically stained slightly silty fine SAND (SP-SM)	77 W	-				
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DRILLER:	Complete Development
EQUIPMENT:	Trackhoe
HOLE DIM.:	
REMARKS:	
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THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-11

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

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P		ZEOEZ	E	D E N	T Y P E	Dry Density (pcl)	Moisture Content %
(U)	SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW.	D	(A) - 0.0 -	ŽŤ	E	λΩ G	28
	2 inches of Topsoil Gray to brown slightly silty fine SAND ("Earthfill", SP-SM)	- 💥	0.0				
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-	Dark brown PEAT with roots (PT)						
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	Dark brown organically stained slightly silty fine SAND (SP-SM)						
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DRILLER: Complete Development

EQUIPMENT: Trackhoe

HOLE DIM.: REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

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EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

TEST PIT NO.: TP-12

D C	OIL CLASSIFICATION		E	S.		IPLE	
p	AND REMARKS	THGHZO	E	g	T	Dry Density (pcl)	Moisture Content %
ar I	YMBOL SHEET FOR EXPLANATION OF DLS AND ABBREVIATIONS BELOW.	n	(f)	D E N	YPE	(F)	Xois Sonte
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2 inches of Topsoil Brown slightly silty fine SAND ("Earthfill"	, SP-SM)						
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Dark brown PEAT with small and large roo	NE (PT)	 	•		88		
Dark brown PEA I with small and range to	, , , , , , , , , , , , , , , , , , ,	\$ 35 F					
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DRILLER: Complete Development EQUIPMENT: Trackhoe

HOLE DIM.: REMARKS:

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE.

PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-13

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

D E P	SOIL CLASSIFICATION	ŗ	E	S	~ -	PLE	S
T	AND REMARKS	E G E	E L E V	DENT	T P E	Dry Density (pcf)	Moisture Content %
(6)	SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW.	N D	(fi)	N T	E	9	X G
É O	2 mehes of Topsoil Brown slightly silty fine SAND with limerock base material ("Earthfill", SP-SM)		- 0.0 -				
A Company of the Comp	Dark brown PEAT with roots (PT)	¥ ±		•			
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10	Dark brown organically stained silty fine SAND (SM)		10.0	The state of the s			mma mannen myteria kritek sartek sartek der diftelijdeldeldelder er er gegene mangen kan de der er men mannen
	Light brown fine SAND (SP) Test Pit Terminated						
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PRILLER Complete Development QUIPMENT: Trackhoe LE DIM,: VARKS:

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SUBSURFACE CONDITIONS AT OTHER
AND AT OTHER TIMES MAY DIFFER.
BEWEEN STRATA ARE APPROXIMATE.
BETWEEN STRATA MAY BE GRADUAL.

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ESA - Brandon Brandon, Florida PROJECT: LOCATION:

TEST PIT NO.: TP-14

EXCAVATED: May 18, 2001 30200-1-9180 PROJ. NO.:

D E	SOIL CLASSIFICATION	L E	E	S		PLES	
P	SOIL CLASSIFICATION AND REMARKS	G	ETEA	- O E N T	T Y	ch Ch	Moisture Content %
H (6)	SEE KEY SYMBOL SHEET FOR EXPLANATION OF SYMBOLS AND ABBREVIATIONS BELOW.	N D	(A)	N T	Ý P E	Dry Density (pcl)	Cont
- ^(ft) -	2 inches of Topsoil Gray to brown slightly silly fine SAND ("Earthfill", SP-SM)	****	- 0.0 -				
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ļ	Brown to dark brown slightly silty fine SAND (SP-SM)						
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- 5 -	Dark brown PEAT with root fibers and wood log (PT)	77 T	5.0				
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-	Dark brown organically stained slightly silty fine SAND (SP-SM)		-				
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DRILLER: EQUIPMENT: HOLE DIM.: REMARKS:	Complete Development Trackhoe

THIS RECORD IS A REASONABLE INTERPRETATION OF SUBSURFACE CONDITIONS AT THE EXPLORATION LOCATION. SUBSURFACE CONDITIONS AT OTHER LOCATIONS AND AT OTHER TIMES MAY DIFFER. INTERFACES BEWEEN STRATA ARE APPROXIMATE. TRANSITIONS BETWEEN STRATA MAY BE GRADUAL.

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PROJECT: ESA - Brandon LOCATION: Brandon, Florida

TEST PIT NO.: TP-15

EXCAVATED: May 18, 2001 PROJ. NO.: 30200-1-9180

KEY TO CLASSIFICATIONS & SYMBOLS

Auger Cuttings	O No Recovery) Dilatometer	Water Table after 24 hours			iller milli A440460		CORRELATION OF PENETRATION RESISTANCE WITH RELATIVE DENSITY AND CONSISTENCY	SILT & CLAY No. of Blows Consistency		5-8 Firm	9-15	16 - 30 Very Stiff Over 30 Hard							KEY TO WINECLU AND	DENCY STORY		CIEC
Undisturbed Sample	Split Spoon Sample	Rock Core	Water Table at time of drilling					CORRELATION OF PEN WITH RELATIVE DENS	SAND & GRAVEL No. of Blows Relative Density	0-4 Ve	5 - 10 Loose		Over 50 Very Dense		IMESTON WS Cor		51 - 50/3" Hard Greater than 50/3" Very Hard				DESC E	V I V III	METOLE
TYPICAL NAMES	Well graded gravels, gravel - sand mixtures, little or no fines.	Poorly graded gravels or grave - sand mixtures, little or no frues.	Silty gravels, gravel - sand - silt mixtures.	Clayey gravels, gravel - sand - clay mixtures.	Well graded sands, gravelly sands, little or no fures.	Poorly graded sands or gravelly sands, little or no fines.	Silty sands, sand - silt mixtures	Clayey sands, sand - clay mixtures.	Inorganic silts and very fine sands, rock flour silty of clayey fine sands or clayey silts and with slight plasticity.	Inorganie lays of low to medium plasticity, gravelly	clays, sandy clays, silty clays, lean clays.	Organic silts and organic silty clays of low plasticity.	Inorganic silts, micaecous or diatomaceous fine sandy or silty soils, clastic silts.	Inorganic clays of high plasticity, fat clays	Organic days of medium to high plasticity, organic silts.	Peat and other highly organic soils.	Lintestone	Weathered Limestone	Soils possessing characteristics of two groups are designated by combinations of group symbols.	GRAVEL Catalog Bouldary	Fine Coarse Country	No.4 3/4" 3" 12" /E SIZE	Engineers, U.S. Аппу Technical , 1960)
GROUP SYMBOLS	MS Can	O. GP	CM CM CM	S	SW	dS	SM SM	SC	M M	5	3	of The	WH	5	OH.	Td 77 7	S H	MLS WLS	ing charact of group s		Medium Coarse	No.10 N RD SIEVE	ı, Corps of vised Apri
		CIKA VELS 6 (Little or no fines)	GRAVELS SWITH FINES	(Appreciable anount of fires)	CLEAN	(Little or no fines)	SANDS WITH	(Appreciable amount of fines)		SILTS AND CLAYS	(Liquid limit LESS than 50)			SILTS AND CLAYS (Liquid liquid GREATER than 50)						SAND	Fine Me	Na.200 No.40 No.10 No.4 U.S. STANDARD SIEVE SI	Reference: The Unified Soil Classification System, Corps of En Memorandum No. 3-357, Vol. , March, 1953 (Revised April, 1
MAJOR DIVISIONS		GRAVELS (More than 50% of	CARGER than the			SANDS (More than 50% of	SWALLER from the No. 4 Sieve Size)			SILTS AN	(Liquid limit)			SILTS AN		HIGHLY ORGANIC SOILS	A Communication of the communi	LIMESTONE FORMATIONS	BOUNDARY CLASSIFICATIONS:		SILT OR CLAY	Z	e Unified Soil Cle Vo. 3-357, Vol. 1
M			COARSE	GRAINED	(More than 50% of material is LARGER	man No. 200 sieve size)				ANNA DE SENSE COM	FINE	GRAINED	(More than 50% of material is	SMALLEK INET ING. 200) sieve size)		HIGH			BOUNDARY C		SIC		Reference: The Memorandum 1

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